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STRUCTURES III

June/July 2016

Time: 3 hours

**THE KENYA NATIONAL EXAMINATIONS COUNCIL****DIPLOMA IN BUILDING TECHNOLOGY****DIPLOMA IN ARCHITECTURE****MODULE III****STRUCTURES III****3 hours****INSTRUCTIONS TO CANDIDATES***You should have the following for this examination:**Answer booklet;**A Scientific calculator.**Answer any FIVE of the following EIGHT questions.**All questions carry equal marks.**Maximum marks for each part of a question are as indicated.**Relevant design tables for this examination are included.**Candidates should answer the questions in English.***This paper consists of 9 printed pages.**

Candidates should check the question paper to ascertain that all the pages are printed as indicated and that no questions are missing.

1. Use the three moments theorem to analyse the loaded beam in figure 1 below; and hence draw the bending moment diagram, indicating values at the critical points. (20 marks)

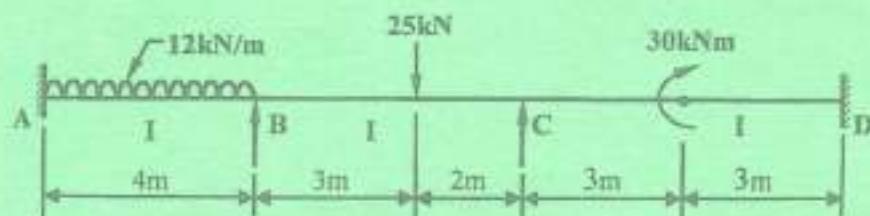


Fig. 1

2. Use moment distribution method to analyse the beam in figure 2 below by making four distributions, and hence draw the bending moment diagram, indicating values at critical points. (20 marks)

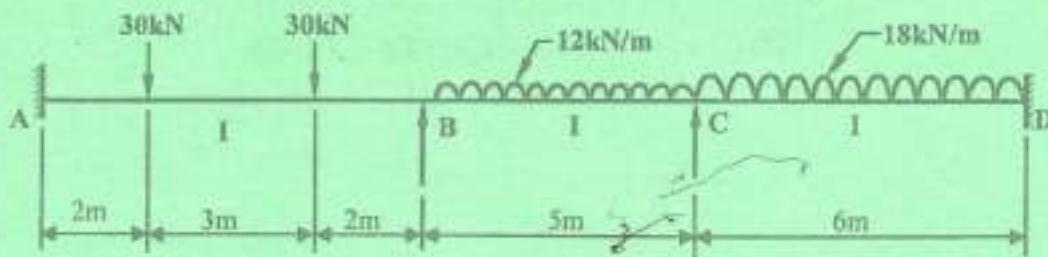


Fig. 2

3. Figure 3 shows a beam with a rolling load moving across it from A to B. Calculate:

- (a) The maximum shear force and corresponding bending moment at point C due to the moving load.
- (b) The maximum bending moment at point C due to the moving load.

(20 marks)

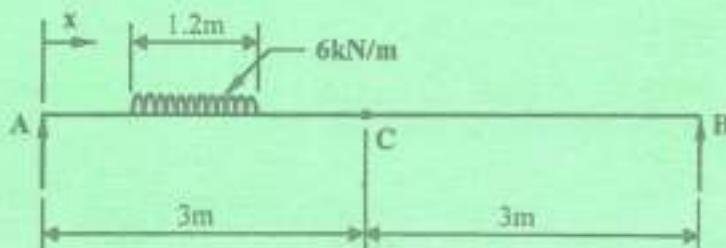


Fig. 3

4. (a) Analyse the beam in figure 4 below, using the three moment theorem and hence draw the bending moment diagram indicating values at critical points.
- (b) Calculate the reactions at supports A, B and C in the figure analysed in (a) above.

(20 marks)

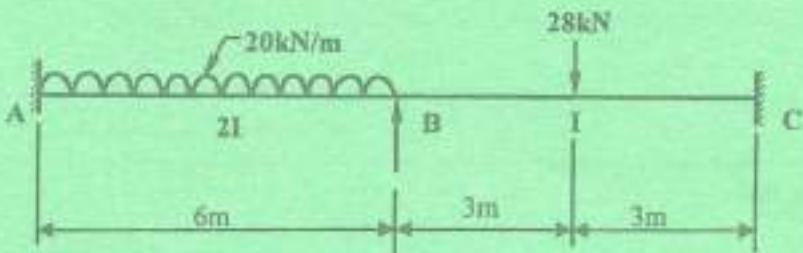


Fig. 4

5. A beam of effective span 6 m is fully restrained laterally and is loaded as described below. Assuming grade 43 steel, check if a 457 x 152 x 67 kg/m universal beam, will be adequate in:

- (a) bending ULS;
 (b) shear ULS;
 (c) deflection SLS.

Data:

- distributed dead load inclusive of self weight = 10 kN/m
- point load at mid-span = 20 kN

Imposed loads:

- distributed load = 14 kN/m
- point load at mid span = 16 kN

(20 marks)

6. (a) Sketch a section of a universal column and indicate the following:
 Flange, web, D, d, T, t.

(4 marks)

- (b) A universal column 4 metres in length between supports, is required to support an axial load of 1100 kN. The ends of the column are restrained in position but not in direction. Check whether a 203 x 203 x 71 kg/m, grade 43 universal column will be adequate.

(16 marks)

7. Use the data provided below to check the adequacy of the joint connection in figure 5 below.

Data:

- 16-mm diameter black bolts grade 4.6 in 18 mm diameter holes
- Take tensile area of bolt as 157 mm^2 and $P_y = 250 \text{ N/mm}^2$.

(20 marks)

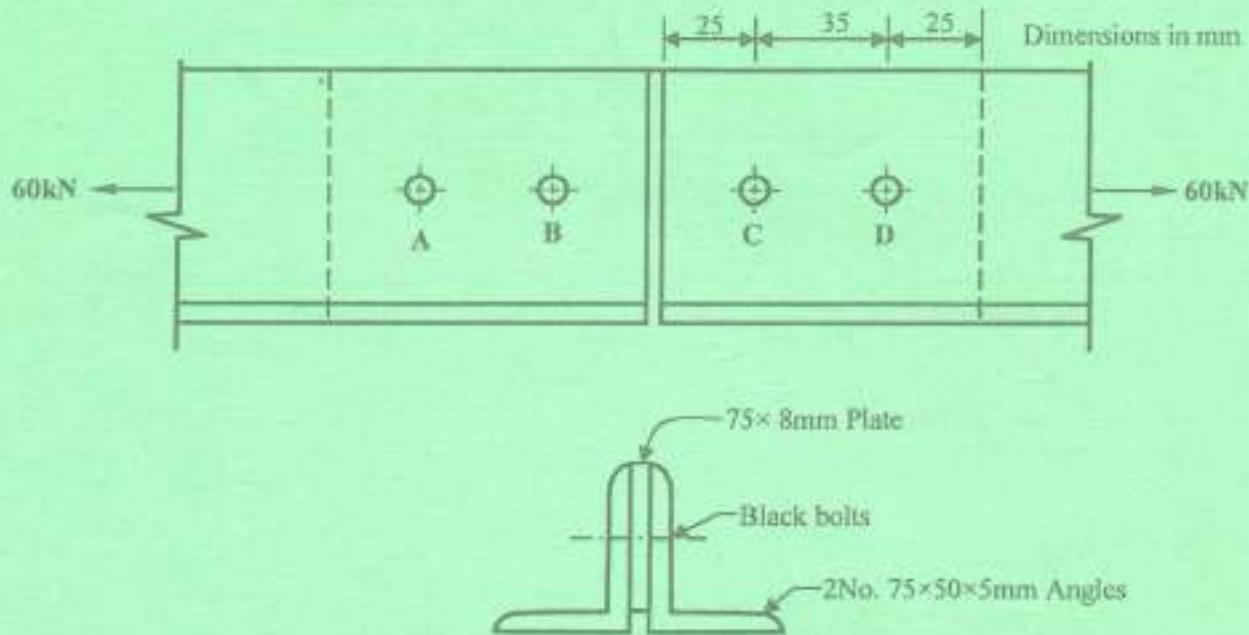


Fig. 5

8. (a) Illustrate a top side notch in timber joists. (2 marks)
- (b) Explain the following terms as used in timber design:
 (i) dry stress;
 (ii) grade stress;
 (iii) glue laminated timber. (6 marks)

- (c) Figure 6 shows the layout of a timber decking support. Using the data provided below, determine the maximum load intensity that can be carried by the joists considering bending and shear stresses only. Assume medium term loading.

Data:

- Size of joist section = 50 x 150 mm
- Grade stresses:
 - bending stress parallel to grain = 12.6 N/mm²
 - shear stress parallel to grain = 1.6 N/mm²

Take:

$$k_3 = 1.25 \quad \text{and} \quad k_7 = 1.08$$

(12 marks)

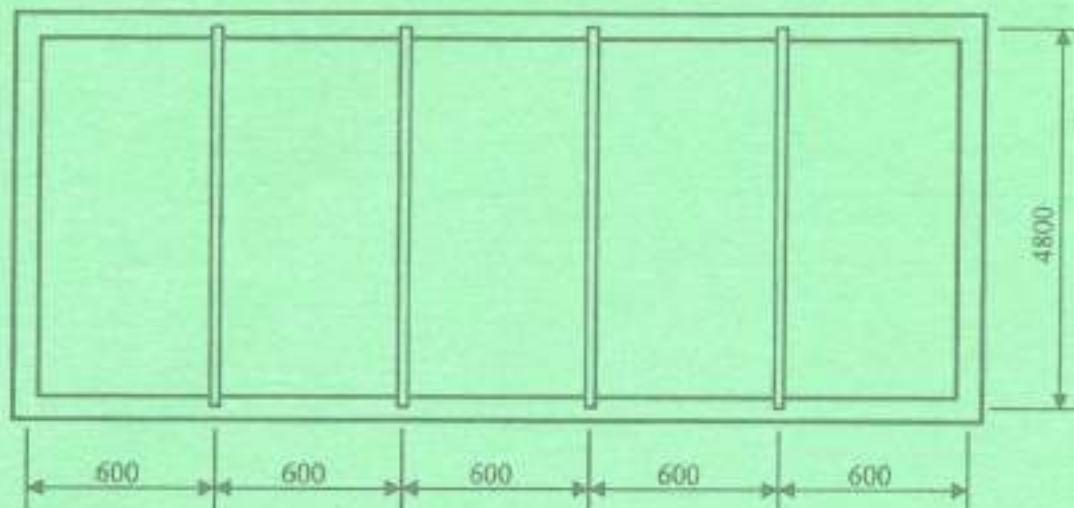


Fig. 6

Note: Dimensions in mm

Table 1 Design strength p_y of grade 43 steel

Thickness less than or equal to (mm)	p_y for rolled sections, plates and hollow sections (N/mm ²)
16	275
40	265
63	255
100	245

The modulus of elasticity E , for deflection purposes, may be taken as 205 kN/mm² for all grades of steel.

Table 2. Ordinary bolts in clearance holes:

<i>Strength of bolts and bearing strength of bolts and connected ply (N/mm²)</i>			
Strength of bolts	Bolt grade		
	4.6	8.8	*
Shear strength p_s	160	375	
Bearing strength p_{sb}	460	1035	
Bearing strength of connected parts	43	50	55
Bearing strength p_u	460	550	650

Table 3. Load capacity (Grade 4.6 bolts and Grade 43 steel)

Nominal diameter (mm)	Shank area A (mm ²)	Tensile stress Area, A_t , A_T (mm ²)
16	201	157
20	314	245
22	380	303
24	452	353

Table 4 Universal beams (abstracted from the *Steelwork Design Guide to BS 5950: Part 1*, published by the Steel Construction Institute)

(a) Dimensions

Serial size (mm)	Designation	Depth of section D (mm)	Width of section B (mm)	Thickness		Root radius r (mm)	Depth between fillets d (mm)	Ratios for local buckling	Dimensions for detailing			Surface area		
				Web t (mm)	Flange T (mm)				Flange b/T %	Web d/t %	End clearance C (mm)	Noch N (mm)	Noch n (mm)	Per metre m ²
355 x 210	122	564.6	213.9	12.8	21.3	12.7	476.5	4.97	37.2	8	110	36	1.89	15.5
	109	539.5	210.7	11.6	18.8	12.7	476.5	5.60	41.1	8	110	32	1.88	17.2
	101	530.7	210.1	10.9	17.6	12.7	476.5	6.04	43.7	7	110	32	1.87	18.3
	92	533.1	209.3	10.3	15.6	12.7	476.5	6.71	48.7	7	110	30	1.86	20.3
	82	528.3	208.7	9.6	13.2	12.7	476.5	7.91	49.4	7	110	26	1.83	22.6
457 x 191	98	467.4	192.0	11.4	19.6	10.2	407.9	4.92	35.8	8	102	30	1.67	17.0
	89	463.6	192.0	10.6	17.7	10.2	407.9	5.43	38.5	7	102	28	1.66	18.6
	82	460.2	191.3	9.9	16.0	10.2	407.9	5.98	41.2	7	102	28	1.65	20.1
	74	457.2	190.5	9.1	14.3	10.2	407.9	6.57	44.3	7	102	26	1.64	22.3
	67	453.6	189.9	8.5	12.7	10.2	407.9	7.48	48.0	6	102	24	1.63	24.4
457 x 152	82	465.1	153.5	10.7	18.9	10.2	407.0	4.06	38.0	7	82	30	1.51	18.4
	74	461.3	152.7	9.9	17.0	10.2	407.0	4.49	41.1	7	82	28	1.50	20.2
	62	457.5	151.9	9.1	15.0	10.2	407.0	5.06	44.7	7	82	26	1.49	22.2
	50	454.7	152.3	8.0	13.3	10.2	407.0	5.75	51.0	6	84	24	1.49	24.3
	32	449.8	152.4	7.6	10.9	10.2	407.0	6.99	53.0	6	84	22	1.48	26.4

Table 5 Universal columns (abstracted from the *Steelwork Design Guide to BS 5950: Part 1*, published by the Steel Construction Institute)

(a) Dimensions

Serial size (mm)	Designation	Depth of section D (mm)	Width of section B (mm)	Thickness		Root radius r (mm)	Depth between fillets d (mm)	Ratios for local buckling	Dimensions for detailing			Surface area		
				Web t (mm)	Flange T (mm)				Flange b/T %	Web d/t %	End clearance C (mm)	Noch N (mm)	Noch n (mm)	Per metre m ²
356 x 406	634	474.7	424.1	47.6	77.0	15.2	290.2	3.75	6.10	26	200	94	1.52	3.88
	551	455.7	418.3	42.0	67.5	15.2	290.2	3.10	6.91	23	200	94	1.48	4.49
	467	436.6	412.4	35.9	58.0	15.2	290.2	3.56	8.08	20	200	74	1.42	5.19
	393	419.1	407.0	30.6	49.2	15.2	290.2	4.14	9.48	17	200	66	1.38	6.05
	340	406.4	403.0	36.3	42.9	15.2	290.2	4.70	11.0	15	200	60	1.35	6.90
COLCORE 477	381.0	399.0	22.6	36.5	15.2	290.2	5.47	12.8	13	200	52	2.31	8.06	
	355	395.0	18.3	30.2	15.2	290.2	6.34	15.7	13	200	46	2.28	9.70	
	302	374.7	374.4	16.8	27.0	15.2	290.2	6.93	17.3	10	190	44	2.19	10.8
	177	368.3	372.1	14.5	23.8	15.2	290.2	7.82	20.0	9	190	40	2.17	12.3
	153	362.0	370.2	12.6	20.7	15.2	290.2	8.94	23.0	8	190	34	2.15	14.1
305 x 305	129	355.6	308.3	10.7	17.3	15.2	290.2	10.3	27.1	7	190	34	2.14	16.6
	283	365.3	321.8	24.9	44.1	15.2	246.6	3.65	9.17	15	158	60	1.94	6.85
	240	352.6	317.9	23.0	37.7	15.2	246.6	4.23	10.7	14	158	54	1.90	7.93
	198	339.9	314.1	19.2	31.4	15.2	246.6	5.00	12.8	13	158	48	1.87	9.45
	158	327.2	310.6	15.7	25.0	15.2	246.6	6.21	15.7	10	158	42	1.84	11.6
254 x 254	137	320.3	308.7	13.8	21.7	15.2	246.6	7.11	17.8	9	158	38	1.82	13.3
	118	314.3	306.8	11.9	18.7	15.2	246.6	8.20	20.7	8	158	34	1.81	5.3
	97	307.8	304.8	9.9	15.4	15.2	246.6	9.90	24.9	7	158	32	1.79	18.4
	167	289.1	264.5	19.2	31.7	12.7	200.3	4.17	10.8	12	154	46	1.58	9.44
	133	276.4	261.0	15.6	25.3	12.7	200.3	5.16	12.8	10	154	40	1.54	11.7
203 x 203	107	266.7	258.3	13.0	20.5	12.7	200.3	6.20	15.4	9	154	34	1.52	14.2
	89	268.4	255.9	10.3	17.3	12.7	200.3	7.40	19.1	7	154	32	1.50	16.9
	73	254.0	254.0	8.6	14.2	12.7	200.3	8.86	23.3	6	154	28	1.49	20.3
	66	222.3	208.8	12.0	20.5	10.2	160.9	5.09	12.4	9	108	32	1.24	16.4
	71	215.9	206.2	10.3	17.3	10.2	160.9	5.96	15.6	7	108	28	1.22	17.2
152 x 152	60	209.6	205.3	9.3	14.3	10.2	160.9	7.23	17.3	7	108	26	1.20	20.3
	52	206.2	203.9	8.0	12.5	10.2	160.9	8.16	20.1	6	108	24	1.19	23.0
	46	203.2	203.2	7.3	11.0	10.2	160.9	9.24	22.0	6	108	22	1.19	25.8
	37	161.8	154.4	6.1	11.5	7.6	123.5	6.71	15.3	6	84	20	0.912	24.6
	30	157.3	152.9	6.6	9.4	7.6	123.5	8.13	18.7	5	84	18	0.9	30.0
	23	152.4	152.4	6.1	6.8	7.6	123.5	11.2	20.2	5	84	16	0.889	38.7

Table 4. Universal beams continued (abstracted from the Steelwork Design Guide to BS 5950: Part 1, published by the Steel Construction Institute)

(b) Properties

Serial size (mm)	Designation	Second moment of area		Radius of gyration		Elastic modulus		Plastic modulus		Buckling parameter	Torsional index	Warping constant	Torsional constant	Area of section A
		Mass per metre (kg/m)	(mm ²)	(mm ²)	(mm)	(mm)	(mm ²)	(mm ²)	(mm ²)					
533 x 210	122	76200	3290	22.1	4.67	2800	320	3200	301	0.876	27.6	2.32	180	156
	109	66700	2940	21.9	4.60	2470	279	2820	435	0.875	30.9	1.89	136	139
	101	61700	2690	21.8	4.56	2300	257	2620	400	0.874	33.1	1.82	103	129
	92	55400	2390	21.7	4.51	2080	229	2370	356	0.872	36.4	1.60	76.2	113
	82	47300	2010	21.3	4.38	1800	193	1600	300	0.865	41.6	1.35	51.3	104
457 x 191	96	49700	2340	19.1	4.13	1960	243	2220	378	0.88	25.8	1.17	121	125
	89	41000	2080	19.0	4.28	1770	217	2010	338	0.879	28.3	1.04	98.5	114
	82	37100	1870	18.8	4.23	1610	196	1830	304	0.877	30.9	0.923	89.2	103
	74	33400	1670	18.7	4.19	1460	173	1660	272	0.876	33.9	0.819	52.0	95.0
	67	29400	1450	18.5	4.13	1300	153	1470	237	0.873	37.9	0.706	37.1	85.4
457 x 152	83	36200	1140	18.6	3.31	1560	149	1800	235	0.872	27.3	0.569	89.3	104
	74	32400	1010	18.5	3.26	1410	133	1620	209	0.87	30.0	0.499	66.6	95.0
	67	28600	878	18.3	3.23	1250	116	1420	182	0.867	33.6	0.429	47.5	83.4
	59	25300	794	18.3	3.23	1120	104	1280	163	0.869	37.5	0.387	33.6	75.9
	52	21300	645	17.9	3.11	949	94.5	1090	133	0.859	43.9	0.341	21.3	66.5

Table 5. Universal columns continued (abstracted from the Steelwork Design Guide to BS 5950: Part 1, published by the Steel Construction Institute)

(b) Properties

Serial size (mm)	Designation	Second moment of area		Radius of gyration		Elastic modulus		Plastic modulus		Buckling parameter	Torsional index	Warping constant	Torsional constant	Area of section A	
		Mass per metre (kg/m)	(mm ²)	(mm ²)	(mm)	(mm)	(mm ²)	(mm ²)	(mm ²)						
356 x 406	634	275600	96200	18.5	11.0	11600	4630	14200	7110	0.843	5.46	38.8	13200	306	
	531	227600	82700	18.0	10.9	9620	3920	12100	6069	0.841	6.05	31.1	9240	103	
	467	183000	67900	17.5	10.7	8390	3290	10000	5049	0.839	6.86	24.3	5320	795	
	393	147000	55400	17.1	10.5	7000	2720	8230	4160	0.837	7.86	19.0	3350	501	
	340	122000	46800	16.8	10.4	6030	2320	6990	3540	0.836	8.85	15.5	2340	433	
287 x 100	100000	38700	16.5	10.3	5080	1940	5820	2950	3035	10.2	12.3	1440	306		
	235	79100	31000	16.2	10.2	4150	1570	4690	2380	0.834	12.1	9.54	812	300	
COLCORE 477	172000	48100	16.8	10.6	3000	3210	9700	4980	0.815	6.91	23.8	5700	607		
	356 x 368	202	56300	23600	16.0	9.57	3540	1260	3990	1920	0.844	13.3	7.14	560	258
	177	57200	20300	15.9	9.53	3100	1100	3460	1670	0.844	15.0	6.07	383	226	
	155	48500	17200	15.8	9.46	2580	944	2940	1420	0.844	17.0	5.09	251	195	
	139	40200	14800	15.6	9.39	2260	790	2480	1209	0.843	19.9	4.16	153	165	
305 x 305	283	78800	24500	14.8	8.25	4310	1530	5100	2340	0.855	7.65	4.33	2630	360	
	240	64200	20200	14.5	8.14	3640	1270	4220	1920	0.854	8.73	3.01	1270	306	
	198	50300	16200	14.2	8.02	2970	1030	3440	1380	0.854	10.2	2.86	734	252	
	158	39700	12500	13.8	7.89	2370	806	2680	1230	0.853	12.5	2.36	379	301	
	137	33800	10700	13.7	7.82	2050	601	2300	1030	0.851	14.1	2.38	230	175	
254 x 254	118	27600	9810	13.6	7.75	1760	587	1920	592	0.851	16.2	1.97	160	150	
	97	22300	7270	13.4	7.68	1440	477	1590	723	0.850	19.3	1.55	91.1	123	
	167	29900	9300	11.9	6.79	2970	761	2420	1136	0.852	8.49	1.62	625	212	
	132	22600	7230	11.6	6.67	1630	576	1870	879	0.850	10.3	1.18	322	188	
	107	17300	5900	11.3	6.57	1310	457	1490	695	0.848	12.4	0.894	173	137	
203 x 283	89	14300	4830	11.2	6.52	1100	379	1230	573	0.849	14.4	0.716	104	114	
	73	11400	3870	11.1	6.44	996	305	989	652	0.848	17.3	0.557	57.3	92.3	
	66	9460	3120	9.27	5.32	831	299	929	458	0.815	10.2	0.317	138	110	
	71	7650	2560	9.16	5.29	708	246	803	374	0.815	11.3	0.25	82.3	91.1	
	60	6090	2040	8.56	5.19	531	199	632	303	0.817	14.1	0.195	46.6	75.3	
152 x 152	52	5260	1770	8.50	5.16	510	174	568	264	0.848	15.8	0.166	32.0	66.4	
	46	4560	1540	8.31	5.11	449	151	497	230	0.846	17.7	0.142	22.2	58.8	
	31	3220	709	6.84	3.87	276	91.2	310	140	0.542	13.3	0.04	19.3	47.4	
	30	3140	538	6.75	3.82	271	73.1	347	111	0.548	16.0	0.036	10.3	38.2	
	23	1360	403	6.31	3.69	166	52.9	184	30.9	0.537	20.4	0.024	4.87	29.3	

Table 6 — Compressive strength $P_{c,y}$ a) Values of p_c (N/mm 2) with $\lambda < 110$ for strut curve b

A	Steel grade and design strength p_y (N/mm 2)															
	S 275					S 355					S 460					
	335	345	355	365	375	310	320	330	340	350	360	370	380	390	400	410
10	235	245	255	265	275	315	325	335	345	355	365	375	385	395	405	415
20	234	243	253	263	273	310	320	330	340	350	360	370	380	390	400	410
25	229	239	249	259	267	304	314	323	333	342	354	363	371	381	390	400
30	225	234	243	253	263	296	307	316	325	335	345	354	362	371	380	389
35	220	229	238	247	256	291	300	318	327	336	347	354	362	370	379	387
40	216	224	233	241	250	284	293	301	310	319	325	334	340	348	354	364
42	213	222	231	239	248	281	288	298	306	314	321	329	335	343	350	359
44	211	220	228	237	245	278	286	294	302	310	318	326	334	342	352	352
46	209	218	226	234	242	275	283	291	299	306	314	322	330	338	346	356
48	207	215	223	231	239	271	279	287	294	302	310	318	326	334	342	350
50	205	213	221	229	237	267	275	283	290	298	306	314	322	330	338	347
52	203	210	218	226	234	264	271	278	286	293	301	309	317	324	331	341
54	200	208	215	223	230	260	267	274	281	288	295	302	309	317	324	334
56	198	205	213	220	227	256	263	270	278	285	292	299	306	313	320	326
58	195	202	210	217	224	262	268	275	281	288	295	302	309	316	323	330

Table 7 — Compressive strength $P_{c,y}$ a) Values of p_c (N/mm 2) with $\lambda < 110$ for strut curve c

A	Steel grade and design strength p_y (N/mm 2)															
	S 275					S 355					S 460					
	335	345	355	365	375	310	320	330	340	350	360	370	380	390	400	410
10	235	245	255	265	275	315	325	335	345	355	365	375	385	395	405	415
20	233	242	252	262	271	308	317	326	335	345	357	366	374	384	394	404
25	228	236	245	254	263	299	308	317	326	335	344	354	362	371	380	389
30	220	228	237	246	255	298	307	315	324	332	341	350	358	366	373	382
35	213	221	230	238	247	280	288	296	305	313	320	328	337	345	353	367
40	206	214	222	230	238	270	278	286	294	302	310	318	326	334	342	350
42	203	211	219	227	235	266	273	281	288	296	304	312	320	328	336	344
44	200	208	215	224	231	263	269	276	284	291	299	306	314	321	329	336
46	197	205	213	220	228	257	264	271	279	286	293	301	309	317	325	337
48	195	203	209	217	224	253	260	267	274	281	289	297	304	311	319	327
50	192	199	206	213	220	248	255	262	269	275	282	289	296	303	310	317
52	189	196	203	210	217	244	250	257	263	270	277	283	290	297	303	310
54	186	193	199	206	213	239	246	252	258	264	271	278	285	292	299	306
56	183	189	196	203	210	234	240	246	252	258	264	270	276	282	289	295
58	179	186	193	199	206	229	235	241	247	253	259	267	273	280	287	293
60	175	182	189	195	201	220	226	232	238	244	250	257	264	271	278	285
62	173	179	185	191	197	219	225	230	235	241	246	251	257	263	270	276
64	170	176	182	188	193	215	220	225	230	235	240	245	250	255	260	266
66	167	173	179	184	189	210	215	220	224	229	234	238	243	248	252	256
68	164	169	175	180	185	205	210	214	219	223	228	231	236	240	244	248
70	161	166	171	176	181	200	204	209	213	217	221	224	228	234	238	244
72	157	163	168	172	177	195	199	203	207	211	217	221	227	231	237	240
74	154	159	164	169	173	190	194	198	202	206	210	214	218	222	226	230
76	151	156	160	165	169	185	189	193	197	200	214	217	220	223	226	230
78	148	153	157	161	165	180	184	187	191	194	207	210	215	217	222	225

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