

To Scan

2707/302
STRUCTURES III
Oct./Nov. 2018
Time: 3 hours



THE KENYA NATIONAL EXAMINATIONS COUNCIL

DIPLOMA IN CIVIL ENGINEERING

MODULE III

STRUCTURES III

3 hours



INSTRUCTIONS TO CANDIDATES

You should have the following for this examination:

- Answer booklet;*
- Scientific calculator.*

This paper consists of EIGHT questions.

Answer any FIVE questions.

All questions carry equal marks.

Maximum marks for each part of a question are as indicated.

Relevant design tables are attached.

Candidates should answer the questions in English.

This paper consists of 9 printed pages.

Candidates should check the question paper to ascertain that all the pages are printed as indicated and that no questions are missing.

1. A simply supported beam is loaded with factored loads as shown in Figure 1. Assuming that the beam is fully laterally restrained, select a suitable universal beam section in grade S 275 (grade 43) steel to satisfy bending, shear and deflection.

Given: Permissible deflection = $\text{span}/200$

$E = 205 \text{ kN/mm}^2$.

(20 marks)

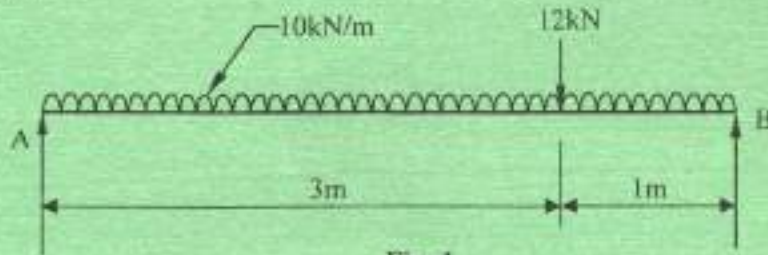


Fig. 1

2. A timber truss with pin-jointed members, carries loads as shown in Figure 2. Using a rectangular timber section of strength class C 16, design member AF and check for slenderness and buckling.

Take the permissible stress as 2.56 N/mm^2 .

(20 marks)

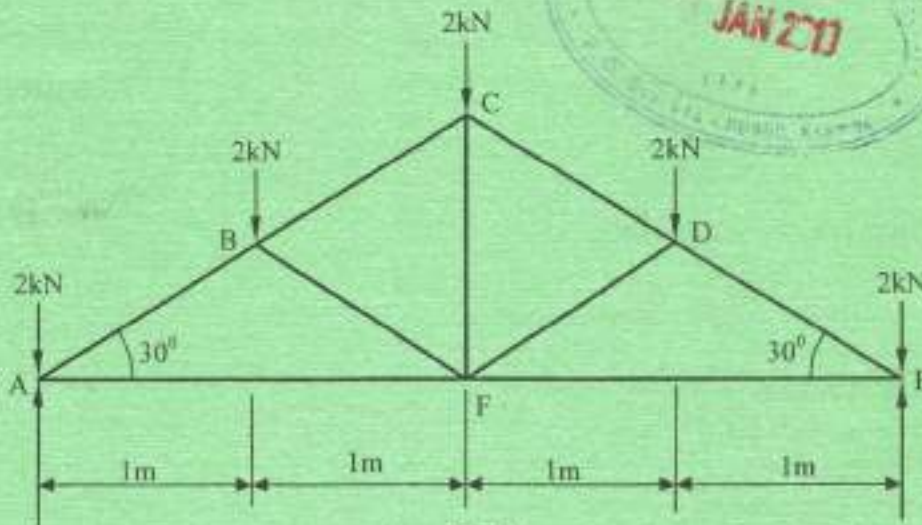


Fig. 2

3. Using the three moments theorem, analyze the beam shown in Figure 3 and sketch the bending moment diagram, indicating the values at all critical points.

(20 marks)

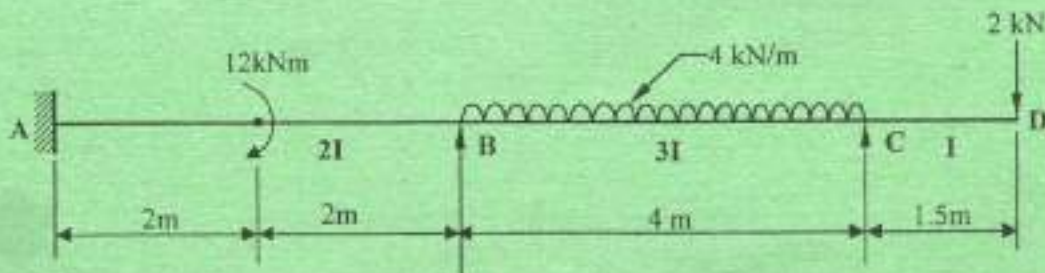


Fig. 3

4. Analyze the figure shown in Figure 4 using the moment distribution method and hence sketch the bending moment diagram, indicating the values at all critical points. (20 marks)

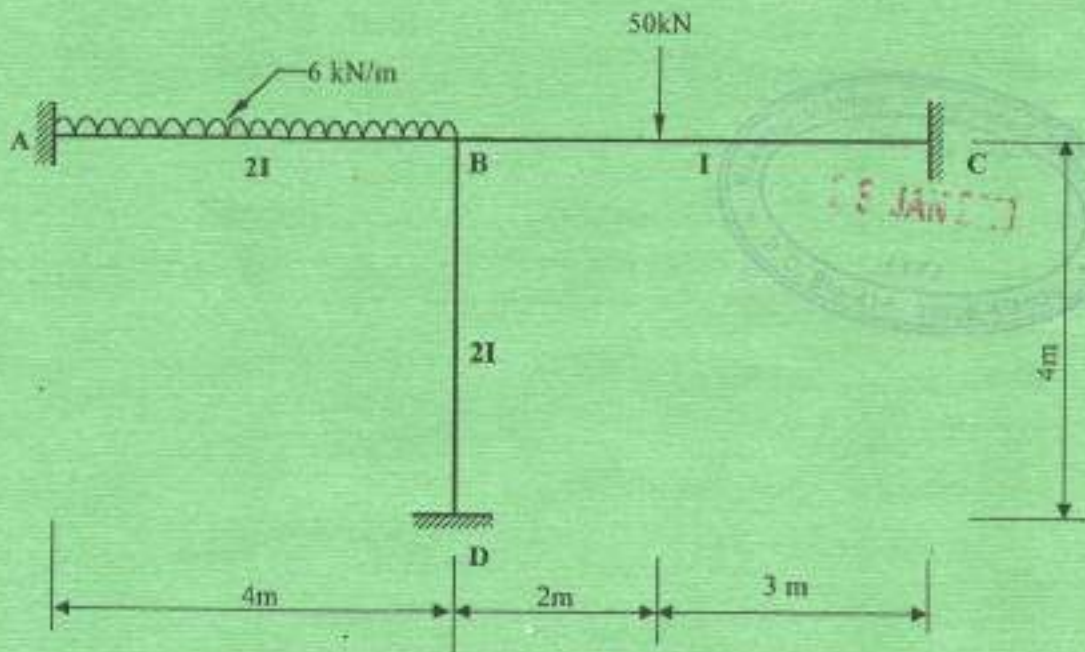


Fig. 4

5. (a) A timber strut of cross section 100 x 50 mm has an effective length of 1.5 m. Calculate the:
- least radius of gyration;
 - slenderness ratio. (6 marks)
- (b) A 3 m long strut with both ends hinged, has a cross section as shown in Figure 5. Using Euler's formula, calculate the critical load that the strut can carry.

$E = 200 \text{ kN/mm}^2$

(14 marks)

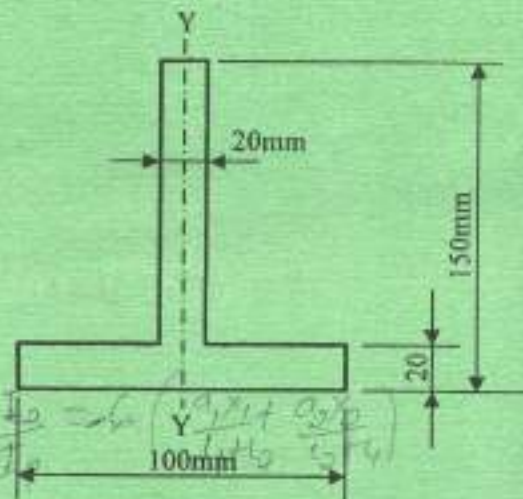
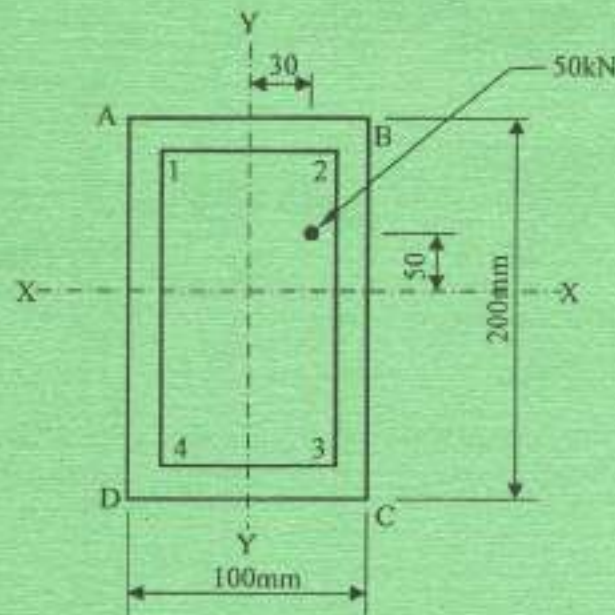


Fig. 5

$$N \left(\frac{1}{I_1} + \frac{2M_B}{I_2} \right) + N \left(\frac{1}{I_1} + \frac{2M_B}{I_2} \right) + N \left(\frac{1}{I_1} + \frac{2M_B}{I_2} \right) = \frac{N}{I_1} + \frac{2M_B}{I_2} + \frac{N}{I_1} + \frac{2M_B}{I_2} + \frac{N}{I_1} + \frac{2M_B}{I_2}$$

6. A uniformly distributed load of 5 kN/m and length 10 m rolls across a simply supported girder of span 8 m. Sketch diagrams of the following load components, plotting the values at 2 m intervals:
- maximum negative shear force;
 - maximum positive shear force;
 - maximum bending moment.
- (20 marks)
7. A hollow steel rectangular column of 10 mm uniform thickness carries a vertical load P of 50 kN at eccentricities of 50 mm and 30 mm about the $x-x$ and $y-y$ axes respectively as shown in Figure 6. Calculate the stresses at the following points:
- external corners B and D;
 - internal corners 1 and 3.
- (20 marks)



8. A steel column of overall length 4 m carries unfactored imposed loads as shown in Figure 7. It is fixed in position and direction at both ends. Select a suitable universal column section in grade S 275 (grade 43) steel and check its adequacy. (20 marks)

Take factored load as 1.6 and assume effective length of the column as 0.7 actual length.

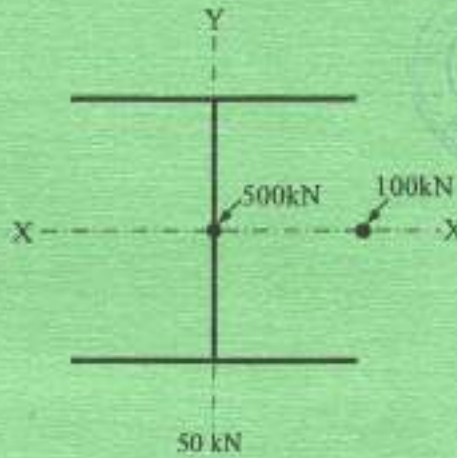
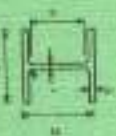
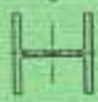


Fig. 7

UNIVERSAL COLUMNS

MS 300-1: 2000
MS 4-1: 1983



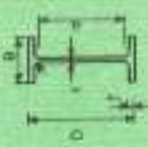
DIMENSIONS

Section Designation	Mass per metre	Depth of Section		Thickness		Radius		Second Moment of Area		Radius of Gyration		Elastic Modulus		Plastic Modulus		Buckling Parameter	Formal Index	Warping Constant	Area of Section
		D	B	Web	Flange	Root	Flange	Axis x-x	Axis y-y	Axis x-x	Axis y-y	Axis x-x	Axis y-y	Axis x-x	Axis y-y				
350x400x20	633.9	474.0	424	47.5	77	15.2	290.2	5.1	279000	59100	16.4	11	11600	4530	14300	7110	0.543	5.48	808
300x400x25	551	455.0	418.5	42.1	67.5	15.2	290.2	3.1	227000	62700	16	10.8	8960	3650	12100	6060	0.441	6.05	702
350x400x17	487	436.0	412.2	35.8	58	15.2	290.2	3.55	193000	67800	17.5	10.7	8360	3220	10000	5030	0.505	6.85	595
350x400x20	503	419	407	30.6	48.2	15.2	290.2	4.14	147000	59400	17.1	10.5	7000	2720	8220	4100	0.337	7.85	507
350x400x30	595.9	436.4	407	28.5	42.9	15.2	290.2	4.7	120000	48900	18.8	10.4	6030	2350	7000	3540	0.336	8.85	433
350x400x20	397.1	303.6	309	22.0	36.5	15.2	290.2	5.47	99000	38700	10.3	10.3	6070	1940	5010	2850	0.325	10.2	366
350x400x25	425.1	301	304.6	18.4	30.2	15.2	290.2	6.34	79100	31050	10.3	10.2	4150	1570	4090	2330	0.334	12.1	299
350x400x20	201.8	374.0	374.7	16.5	27	15.2	290.2	6.94	68300	23700	16.1	9.8	3540	1260	3970	1920	0.44	13.4	307
350x400x17	177	368.2	373.8	14.4	23.8	15.2	290.2	7.83	57100	20500	16.9	9.54	3100	1060	3480	1670	0.444	15	226
350x400x15	152.8	302	370.5	12.3	20.7	15.2	290.2	8.95	49000	17900	15.8	9.49	2880	948	3060	1430	0.444	17	195
350x400x12	129	253.8	308.0	10.4	17.8	15.2	290.2	10.5	40000	14600	15.2	9.43	2260	702	2480	1200	0.544	19.9	164
300x300x28	282.8	365.3	322.2	26.8	44.1	10.2	248.7	3.65	78900	24600	14.6	8.27	4320	1530	5110	2340	0.605	7.65	360
300x300x20	240	392.8	318.4	23	37.7	10.2	248.7	4.23	54200	20300	14.8	8.15	3640	1260	4250	1900	0.654	8.74	306
300x300x18	198.1	339.3	314.5	18.1	31.4	10.2	248.7	5.01	50800	18300	14.2	8.04	3000	1040	3440	1580	0.854	10.2	252
300x300x15	159.1	327.1	311.2	15.8	28	10.2	248.7	6.22	38700	12900	13.9	7.9	2370	808	2680	1230	0.851	12.5	201
300x300x13	138.0	300.5	300.2	13.8	21.7	10.2	248.7	7.12	32900	10700	13.7	7.83	2050	692	2300	1050	0.851	14.2	174
300x300x11	117.8	314.5	307.4	12	18.7	10.2	246.7	8.22	27700	9000	13.6	7.77	1760	589	1960	895	0.85	16.2	150
300x300x8	99.8	307.3	305.3	9.6	15.4	10.2	246.7	9.81	22200	7310	13.4	7.85	1450	479	1590	736	1.58	19.3	123
250x250x16	107.1	289.1	293.2	19.2	31.7	12.7	200.3	4.10	30000	9070	11.9	6.81	2080	744	2420	1140	0.801	8.40	273
250x250x12	132	276.3	281.3	15.3	25.3	12.7	200.3	5.16	22500	7500	11.6	6.80	1430	576	1870	978	0.85	10.3	188
250x250x10	107.1	266.7	258.8	12.6	20.9	12.7	200.3	6.31	17900	5300	11.2	6.99	1310	459	1480	687	0.848	12.4	156
250x250x8	88.9	263.3	256.3	10.3	17.5	12.7	200.3	7.41	14300	4880	11.2	6.95	1100	379	1220	575	0.85	14.5	113
250x250x7	75.1	264.1	254.8	8.8	14.2	12.7	200.3	8.96	11400	3810	11.1	6.48	898	307	920	485	0.840	17.3	83.1
200x200x8	80.1	222.2	208.1	13.7	20.5	10.2	160.8	5.1	9450	3130	9.26	5.34	650	230	977	450	0.80	10.2	110
200x200x7	71	215.8	206.4	10	17.5	10.2	160.8	6.97	7600	2540	8.18	5.2	700	246	795	374	0.803	11.9	90.4
200x200x6	60	209.0	205.8	8.4	14.2	10.2	160.8	7.35	6130	2060	8.85	5.2	584	201	656	305	0.848	14.1	76.4
200x200x5	52	206.2	204.3	7.9	12.5	10.2	160.8	8.17	5280	1780	8.91	5.16	510	174	567	264	0.848	15.8	66.5
200x200x4	46.1	203.2	203.6	7.2	11	10.2	160.8	9.25	4570	1560	8.80	5.13	450	152	407	231	0.847	17.7	59.7
150x150x17	97	161.8	154.4	8	11.5	7.6	123.6	6.71	2210	706	6.63	3.87	270	97.5	309	140	0.548	13.3	47.1
150x150x15	80	157.6	152.0	6.5	9.4	7.6	123.6	8.10	1740	560	6.76	3.83	222	73.3	246	112	0.840	16	36.3
150x150x12	66	152.4	152.2	5.6	8.8	7.6	123.6	11.2	1250	400	6.54	3.7	164	52.8	182	80.1	0.84	20.7	29.2

UNIVERSAL BEAMS



US 598541-2009
BS 6-1-1983



DIMENSIONS

PROPERTIES

Section Designator	Mass per Metre Weight	Depth of Suction	Width of Suction	Thickness		Depth of Flange	Depth of Flange	Radius of Gyration		Elastic Modulus	Plastic Modulus		Buckling Factor	Torsional Index	Welding Coefficient	Torsional Coefficient	Area of Section
				Web	Flange			Axis x-x	Axis y-y		Axis x-x	Axis y-y					
457 x 191 x 68	98.3	407.2	162.8	11.4	10.6	10.2	407.8	15.1	4.30	1965	243	2230	0.882	25.7	1.18	1.97	126
457 x 181 x 66	98.3	403.4	161.9	10.5	17.7	10.2	407.6	54.1	36.8	1775	218	2070	0.878	28.5	1.04	1.91	114
457 x 161 x 62	82.0	405.0	161.3	9.8	16.0	10.2	407.6	5.98	41.3	1835	234	1835	0.878	30.8	0.822	1.85	156
457 x 151 x 58	74.8	407.0	160.4	14.5	14.5	10.2	407.6	6.57	45.3	1820	198	1850	0.877	33.8	0.818	1.81	146
457 x 131 x 54	67.1	403.4	159.3	8.5	12.7	10.2	407.6	4.48	49.0	1820	158	1870	0.877	37.9	0.799	1.71	105.5
457 x 122 x 62	61.1	405.8	159.5	10.5	18.9	10.2	407.6	4.11	33.7	1820	153	1910	0.877	27.4	0.947	1.82	102
457 x 107 x 54	54.2	405.0	154.4	8.6	17.0	10.2	407.6	4.54	33.9	1820	118	1820	0.873	30.2	0.918	1.81	101
457 x 102 x 67	67.9	405.0	155.0	9.9	19.0	10.2	407.6	5.13	45.3	1820	118	1820	0.868	33.9	0.848	1.71	101.0
457 x 102 x 65	64.8	405.8	152.8	8.1	13.3	10.2	407.6	5.75	50.3	1820	118	1820	0.868	37.5	0.847	1.68	101.0
457 x 102 x 55	58.2	405.8	153.4	7.6	10.8	10.2	407.6	6.81	31.8	1820	118	1820	0.868	43.8	0.811	1.68	101.0
457 x 102 x 52	54.2	405.8	153.4	7.6	10.8	10.2	407.6	6.81	31.8	1820	118	1820	0.868	43.8	0.811	1.68	101.0
457 x 102 x 48	48.0	405.8	153.4	6.4	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 45	44.0	405.8	153.4	6.4	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 42	40.0	405.8	153.4	6.4	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 39	36.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 36	32.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 33	28.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 30	24.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 27	20.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 24	16.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 21	12.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 18	8.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 15	4.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0
457 x 102 x 12	0.1	405.8	153.4	6.0	10.2	10.2	407.6	6.36	33.0	1820	118	1820	0.868	47.5	0.785	1.68	101.0

Table 9: Design strengths, p_y , for steel

Design Grade	Thickness (mm), less than or equal to	Sections, plates and hollow sections, p_y (N/mm ²)
43	16	275
	40	265
	63	255
	100	245
50	16	355
	40	345
	63	340
	100	325
55	16	450
	25	430
	40	425
	63	400

Table 11 — Limiting width-to-thickness ratios for sections other than CHS and RHS

Compression element		Ratio ^a	Limiting value ^b		
			Class 1 plastic	Class 2 compact	Class 3 semi-compact
Outstand element of compression flange	Rolled section	b/T	9E	10E	15E
	Welded section	b/T	8E	9E	13E
Internal element of compression flange	Compression due to bending	b/T	28E	33E	40E
	Axial compression	b/T	Not applicable		
Web of an I- or box section ^c	Neutral axis at mid-depth	d/l	80E	100E	120E
	Generally ^d	If r_1 is negative:	$\frac{80E}{1+r_1}$	$\frac{100E}{1+r_1}$	$\frac{120E}{1+2r_2}$
		If r_1 is positive:	but 40E	$\frac{100E}{1+1.5r_1}$ but 40E	but 40E
	Axial compression ^e	d/l	Not applicable		

^a Dimensions b , d , T and l are defined in Figure 5. For a box section b and T are flange dimensions and d and l are web dimensions, where the distinction between webs and flanges depends upon whether the box section is bent about its major axis or its minor axis, see 3.5.1.

^b The parameter $E = 210 \times 10^3 \text{ N/mm}^2$.

^c For the web of a hybrid section E should be based on the design strength p_y of the flanges.

^d The stress ratios r_1 and r_2 are defined in 3.5.5.



Selected Softwood Timber Sizes (BS 5268)

Geometrical properties of sawn softwoods based on timber with a 20% moisture content

Target Sizes	Area	Section Modulus		Section Moment of Area		Radius of Gyration	
		About x-x	About y-y	About x-x	About y-y	y-y	x-x
mm	10 ³ mm ²	10 ³ mm ³	10 ³ mm ³	10 ⁶ mm ⁴	10 ⁶ mm ⁴	mm	mm
25x50	1.25	10.4	5.2	0.26	0.065	14.4	7.2
25x75	1.88	23.5	7.8	0.88	0.098	21.6	7.2
50x75	3.75	46.9	31.3	1.76	0.781	21.7	14.4
50x100	5.00	83.3	41.7	4.17	1.04	28.9	14.4
50x150	7.50	188	62.5	14.1	1.56	43.3	14.4
50x200	10.0	333	83.3	33.3	2.08	57.7	14.4
75x100	7.50	125	93.8	6.25	3.52	28.9	21.7
75x150	11.3	281	141	21.1	5.27	43.3	21.7
75x200	15.0	500	188	50.0	7.03	57.7	21.7
100x100	10.0	167	167	8.33	8.33	28.9	28.9
100x150	15.0	375	250	28.1	12.5	43.3	28.9
100x200	20.0	667	333	66.7	16.7	57.7	28.9
100x250	25.0	1010	417	130	20.8	72.2	28.9
100x300	30.0	1500	500	225	25.0	86.6	28.9
150x150	20.0	563	563	42.2	42.2	43.3	43.3
150x200	25.5	1000	750	100	56.3	57.7	43.3
150x300	30.0	2250	1130	338	84.4	86.6	43.3

Table 19 — Maximum depth to breadth ratios (solid and laminated members)

Degree of lateral support	Maximum depth to breadth ratio
No lateral support	2
Ends held in position	3
Ends held in position and member held in line as by purlins or tie rods at centres not more than 30 times breadth of the member	4
Ends held in position and compression edge held in line, as by direct connection of sheathing, deck or joists	5
Ends held in position and compression edge held in line, as by direct connection of sheathing, deck or joists, together with adequate bridging or blocking spaced at intervals not exceeding six times the depth	6
Ends held in position and both edges held firmly in line	7

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